

السؤال الثاني (٣٠ درجة)

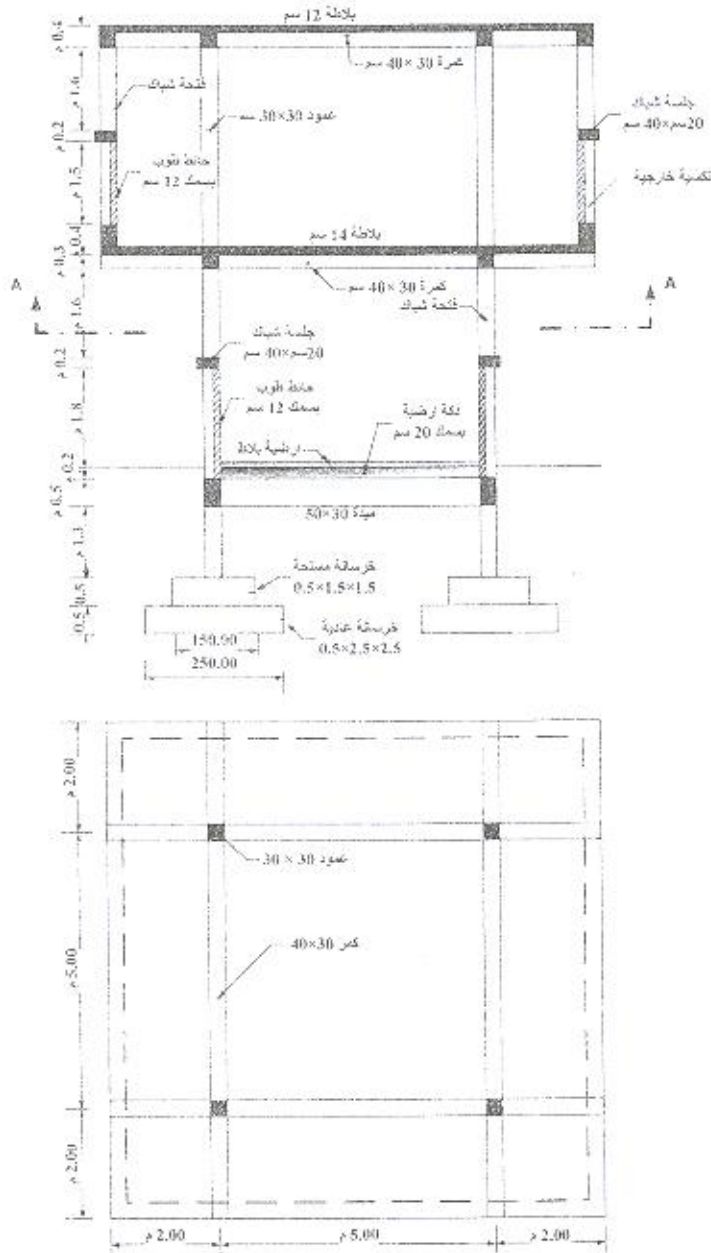
أ- بوصفتك مهندس الاستشاري بالموقع، أحسب الكميات الأتية لتنفيذ عرفة أمن على بوابة إحدى القرى السياحية

علماً بأن الشكل التالي يوضح قطاع افقى لسقف الغرفة بالإضافة الى قطاع رأسي

- i. الحفر (قواعد - سمالات)
- ii. الخرسانة العادية (قواعد خ ع - دكة ارضية - جرسانة الميول للسطح بسبك ٧ سم)
- iii. الخرسانة المسلحة (قواعد خ م - سمالات - عمدان - كمرات - بلاطات)
- iv. حديد التسليح للقواعد علماً بأن حديد تسليح القواعد  $\phi 5$  و  $\phi 12$  فى الاتجاهين
- v. اعمال عزل فقط لقصة الردم

ب- اعداد المقايسة التتمينية الازمة لتنفيذ ماتم حصرة من اعمال

(صمم جدول المقايسة - اقترح صياغة البنود - وضح الكميات للبنود المقترحة بوحدها فى الجدول)



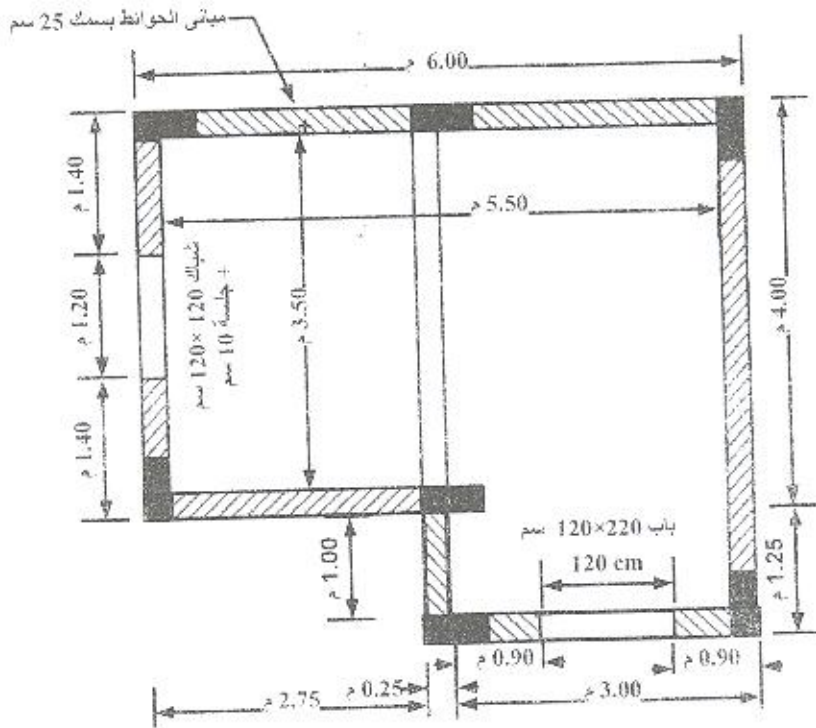
Section A-A

السؤال الثالث: (٢٠ درجة)

- أ- ضع علامة (✓) أمام العبارات الصحيحة وعلامة (×) أمام العبارات الخاطئة:
١. تعتبر ترايبس الخرسانة للممرات من اعمال الخرسانة العادية
  ٢. قد تقاس اعمال الخرسانة المسلحة لاي سمك بالمتر المربع في حالة عمل بند موحد للبلاطات مع الكمرات
  ٣. لا يمكن عمل بند منفصل لحديد التسليح في مقاييسات اصال الترميم
  ٤. تصنيف مشروعات التشييد طبقا لنوعية الاعمال الى مشروعات المباني و مشروعات الهندسة المدنية.
  ٥. من انواع المقاييسات التتمنية مقاييس كمية و مقاييس فعلية
  ٦. مساحة الحفر = الطول الكلي × عرض الكلي للمنشاء من الرسومات
  ٧. الاعتاب من اعمال الخرسانة المسلحة
  ٨. يقاس العمود في الدور الارضي من منسوب ظهر القاعدة حتى منسوب بطنية البلاطة في حالة عدم وجود رقبة عمود
  ٩. تكتب صياغة بنود الخرسانة العادية في المقاييس التتمنية
  ١٠. يمكن جمع اعمال البلاطات و الكمرات للدور في بند واحد

ب- الشكل التالي يبين المسقط الانشائي لغرفة تم بنائها حديثا أعلى عقار. اذ كان ارتفاع الدور انصافي ٣,٠٠ م. قطاعات جميع الكمرات ٥٠ × ٢٥ سم و قطاعات جميع الأعمدة ٦٠ × ٢٥ سم سمك البلاطة ١٠ سم. احسب:

١. كمية المباني اللازمة لعمل الحوائط الخارجية للغرفة
٢. اعمال البياض الداخلي فقط ( بياض المحارة - الدهان)



**السؤال الرابع: (١٥ درجة)**

**أ- اختار الاجابة الصحيحة**

١. الموصفات الفنية هي الجزء الذي
  - (a) يحدد العلاقة التعاقدية و طريقة سداد الدفعات
  - (b) يختص بحساب كميات المواد المستخدمة
  - (c) يختص بشرح المواد الخام و المواد الصنعة المستخدمة
٢. في حالة تعارض الملاحظات الواردة بالرسومات مع الموصفات يتم الأخذ
  - (a) بالملاحظات بالرسومات
  - (b) بالموصفات
  - (c) برأى المهندس الاستشاري
٣. تتطلب مصلحة المالك لضمان حقوقه ان تحتوى الموصفات على
  - (a) شروط قاسية على المقاول
  - (b) شروط متوازنة
  - (c) شروط متساهلة لضمان عدم اعاقة العمل
٤. في حالة استخدام المقاسات او الاشكال الغير قياسية للاقطار
  - (a) ينعكس ذلك على التكاليف النهائية للمشروع
  - (b) لا علاقة له بتكلفة المشروع
  - (c) يمكن للمقاول استبدالها بقطاعات اخرى قياسية دون الرجوع للمهندس
٥. يجب عدم وصف كلا من طريقة التنفيذ و النتائج المطلوبة بالموصفات الفنية
  - (a) لعدم الاطالة
  - (b) لكي لا يشترك المالك في تحمل المسؤولية عن النتائج
  - (c) لكي يشترك المالك في تحمل المسؤولية عن النتائج

ب- ضع علامة (✓) امام العبارات الصحيحة و علامة (×) امام العبارات الخاطئة:

١. تتسبب الموصفات الفنية المعدة جيدا في خفض تكاليف المشروع النهائية على المالك
٢. يتم اضافة عبارة (من احسن صنف) لبيود المواد الخام لضمان جودتها
٣. يستخدم اكثر من مرادف في الموصفات لتأكيد المعنى
٤. يفضل استخدام المقاسات و الاقطار القياسية للمواد
٥. كلما زادت اهمية المشروع كلما ازداد التدقيق في الموصفات و المعلومات الواردة بها
٦. يمكن استخدام موصفات مشروع معدة مسبقا في أى مشروع متشابه معه
٧. الموصفات و الرسومات يكمل كل منهما الاخر
٨. يتم استخدام الضمان في كتابة الموصفات للاختصار
٩. في حالة قيام المالك بعمل الجسات فمن الطبيعي ان يسلمها للمقاول مع نخلة عن مسؤولية دقة المعلومات الواردة بها
١٠. من غير المفضل استخدام صيغة فعل الامر بالموصفات الفنية و يستخدم بدلا منها فعل التفضيل

مع اطيب الاماني بالتوفيق

د. نشوى يوسف و لجنة الممتحنين:



P. El-Sayed El-Sayed  
2011/2012

Dept.: Structural Engrg.	Faculty: Engineering	University : Tanta
Time allowed: 1 hr.	Course: Design of steel structures (a)	Course code: CSE3124
Date: January 2011		

Note:

- It is allowed to use any tables or Egyptian Code of Practice books.
- Any missing data may be reasonably assumed.
- Attempt all questions. Max. Credit 100 % only.
- Number of examination pages: (4).

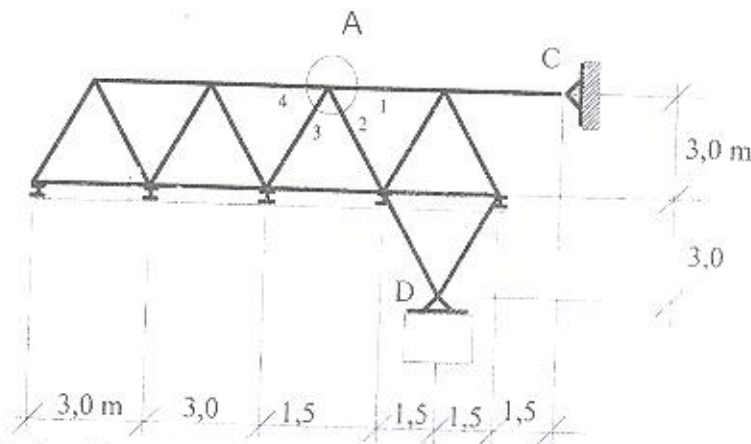
### Question 1:

(25 %)

The steel skeleton of a vehicles-shed is built up of trusses type shown below. The system shown is **two-hinged truss** at points C and D.  
Given the following data, answer the required questions.

Data:

- Spacing between trusses = 4.0 m
- Own weight of steel structure = 30 kg/m<sup>2</sup> of covered area.
- Live load = 60 kg/m<sup>2</sup>
- Weight of cover = 20 kg/m<sup>2</sup>
- Neglect the effect of wind pressure.
- Use steel grade St37.



Note: The cover is fixed at the bottom chord

Required:

- 1) Draw to sketch different views showing the arrangements of bracing system. The length of the covered area is 20.0 m. (10%)
- 2) Calculate the design forces in marked members at joint A. (5%)
- 3) Design an intermediate purlin as rolled steel section. (10%)



TANTA UNIVERSITY  
 FACULTY OF ENGINEERING  
 DEPARTMENT OF STRUCTURAL ENGINEERING

EXAMINATION (THIRD YEAR) STUDENTS OF STRUCTURAL ENGINEERING

COURSE TITLE: DESIGN of REINFORCED CONCRETE STRUCTURES (2) a	COURSE CODE: CSE3123
DATE: January - 2011	TERM: FIRST
TOTAL ASSESSMENT MARKS: 75	TIME ALLOWED: 4 hours

Systematic arrangement of calculations and clear neat drawings are essential. Any missing data can be reasonably assumed. The exam consists of FIVE questions in two pages.

**Problem # One**

(17Marks)

TRY ALL PROBLEMS

- Proof the code equation  $q_{tu} = M_{tu} / 2A_o \times t_e$  for the nominal ultimate torsional shear stress. What is the meaning of the terms "Equivalent thin-walled tube" in torsional analysis? (3Marks)
- Compare between shear and torsion with regard to: stress distribution, mode of failure, code requirements. (3Marks)
- Draw the B.M.D, S.F.D and T.M.D for the beams shown in Fig.(1-a) under the given loads. (4Marks)
- Check design the section shown in Fig. (1-b) subjected to the following actions:  $M_u = 400\text{kN.m}$ ,  $Q_u = 400\text{kN}$ ,  $M_{tu} = 150\text{kN.m}$ .

Materials:  $f_{cu} = 40\text{MPa}$ ,  $f_y = 400\text{MPa}$ . (7Marks)

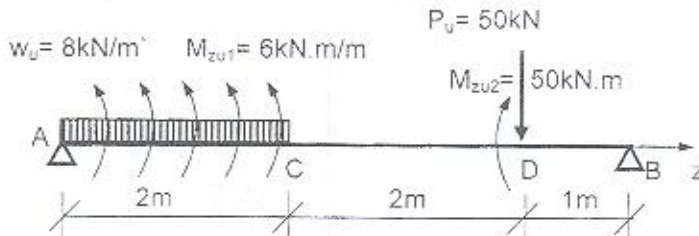


Fig. 1-a

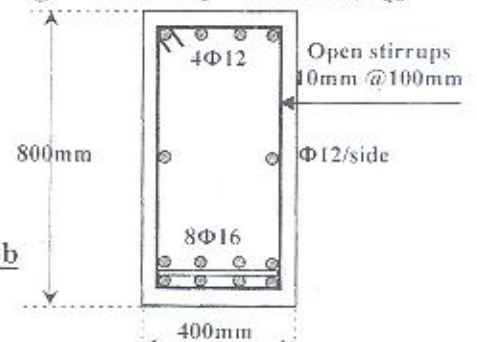


Fig. 1-b

**Problem # Two**

(6Marks)

Fig. 2 shows layout of a first floor resting on eight columns with area of  $8 \times 9.6\text{m}$ . The panelled beams system is required to cover the floor using the beam modules shown in figure. The slab is subjected to L.L =  $5\text{kN/m}^2$  and cover =  $1.5\text{kN/m}^2$ . The slab thickness is 100mm. It is required to make a complete design (design + drawing details) of the panelled beam By1 only.

Materials:  $f_{cu} = 25\text{MPa}$ ,  $f_y = 360\text{MPa}$

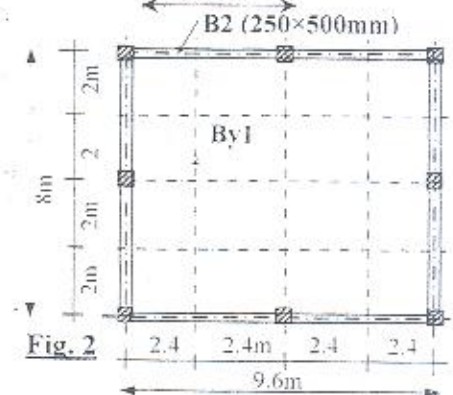


Fig. 2

**Problem # Three**

(24Marks)

- Explain the concept of using the hollow-block slab systems? (3Marks)
- Compare between the solid slabs, hollow block slabs, waffle slabs and flat slabs, with regard to: load transfer, economy and advantages. (3Marks)
- Fig. 3 shows a structural plan of roof ABCD. The roof is resting on the beams (AD and EF) and on the frames (EB and FC). The slab Aefd is dropped 100mm. The hollow-block slab system is required. The slabs is subjected to a live load =  $6\text{kN/m}^2$  and a flooring cover =  $1.8\text{kN/m}^2$ . The cross section of all beams is  $250 \times 600\text{mm}$ . Materials:  $f_{cu} = 40\text{MPa}$ ,  $f_y = 400\text{MPa}$ . It is required to carry out the following:
  - Draw the B.M.D and S.F.D of critical strips. (5Marks)
  - Design the slabs at critical sections. Compute the width of the solid parts due to the S.F and B.M. (6Marks)
  - Draw on plan the reinforcement details and the arrangement of hollow blocks. (4Marks)
  - Compute the load acting on the supporting beam EF. (3Marks)

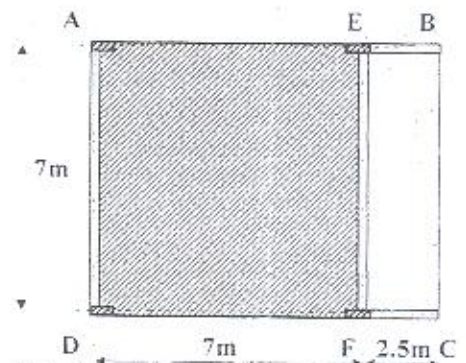


Fig. 3

**Problem # Four**

(22Marks)

(A) Fig. (4-a) shows part of a plan of a typical floor of RC flat slab  $36 \times 40\text{m}$  consists of 25 panels each side 5 panels  $7.2 \times 8\text{m}$  each panel and slab thickness  $0.25\text{m}$  ( $t_s = 250\text{mm}$ ) without drop panel and with column head  $1.50\text{m} \times 1.50\text{m}$ . The flat slab is resting on square columns  $0.5\text{m} \times 0.5\text{m}$ . The marginal beams  $0.3\text{m} \times 0.9\text{m}$  are used at the outer edges of the flat slab. The flat slab is subjected to a uniformly ultimate (factorized) load,  $W_u = 20\text{kN/m}^2$ . Materials:  $f_{cu} = 25\text{MPa}$ ,  $f_y = 360\text{MPa}$ . Using the empirical method of the Egyptian code of practice ECP203-2007 for design of flat slab, it is required to carry out the following:

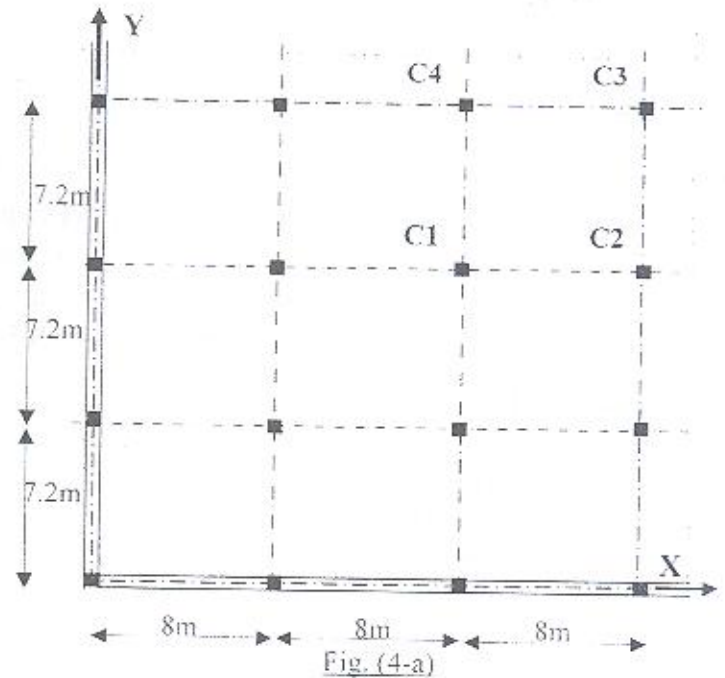


Fig. (4-a)

- i- Determine the critical bending moment in column and field strips in long direction only. (4Marks)
- ii- Design the critical sections due to bending moment in column strip and field strips for the intermediate panel C1 C2 C3 C4 only. (5Marks)
- iii- Check one-way and two-way shear stresses for the interior column C1 considering the case of the total load only. (4Marks)
- iv- Draw on plan the reinforcement details of the column and field strips in the intermediate panel C1 C2 C3 C4 only. (4Marks)

- (B) What will be the punching shear stresses if a large opening exists at a distance 0.4m from the edge of column head of the interior column C1 in the previous problem, as shown in Fig. (4-b), and considering the case of total load only? And what will be the moment transfer from the flat slab to the internal column C1 in this case? (5Marks)

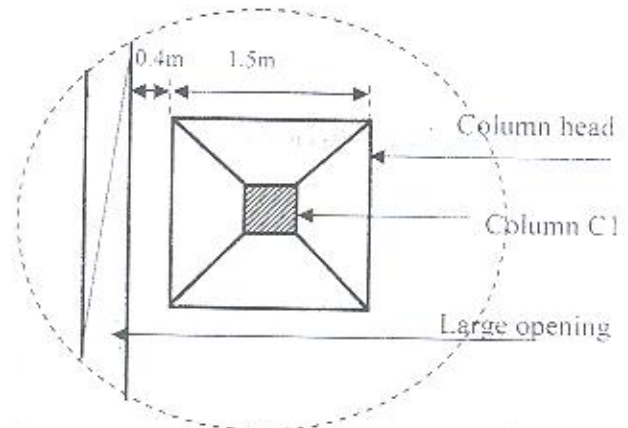
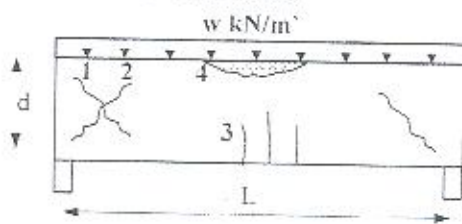


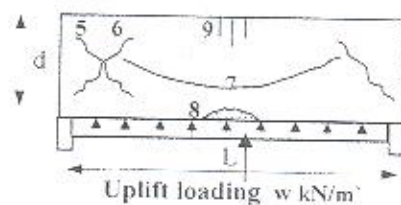
Fig. (4-b). Internal column C1 with large opening at 0.4m from the edge of the column head

**Problem # Five** (12Marks)

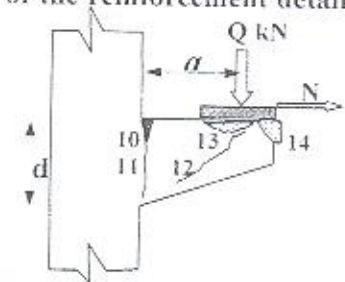
- a. Fig. 5 shows the expected cracks (1 to 14) on a beams and a cantilever under a different case of loadings. What are the reasons of occurrence the cracks shown? How the Egyptian Code equations resisting the cracks shown in case 6 of a corbel? What are the precautions of the reinforcement details of the corbels? (8Marks)



Case 1:  $L/d = 5$   
Case 2:  $L/d = 1.5$



Case 3:  $L/d = 5$   
Case 4:  $L/d = 1.5$



Case 5:  $a/d = 3$   
Case 6:  $a/d = 0.5$

- b. Compare between shallow and deep beams with regard to: strain distribution – limits according to the following codes: ACI (318-2005), ECP-203 2007, Canadian, Euro Code2, BS 8110 (97). What is your opinion about the transition zone from shallow to deep beams? (4Marks)

All the Best

Prof. Dr. Mohamed Kasem Prof. Dr. Tarek El-Shafiey



Remarks: (a .answer the following questions. - b. assume any missing data.)

**1- Problem (1) 15 Marks:**

Using the force method, draw the B.M.D. for the given frame of constant I shown in Fig. (1).

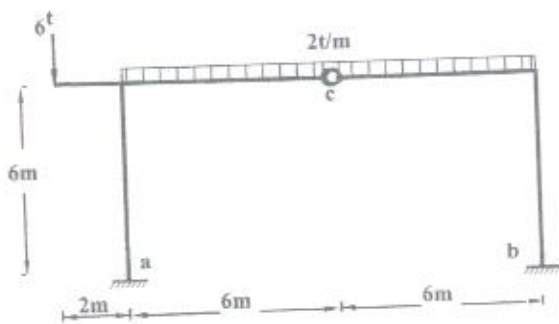


Fig. 1

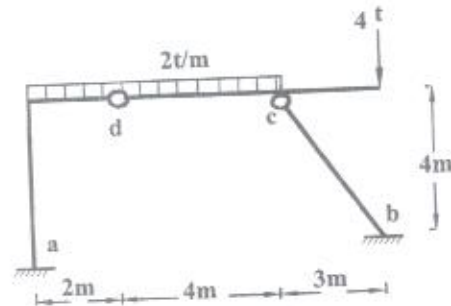


Fig. 2

**2- Problem (2) 15 Marks:**

Using the force method, draw the B.M.D. for the given frame of constant I shown in Fig. (2).

**3- Problem (3) 20 Marks:**

Using the force method, draw the B.M.D. for the given frame of constant I which hinged at a and roller at b shown in Fig. (3).

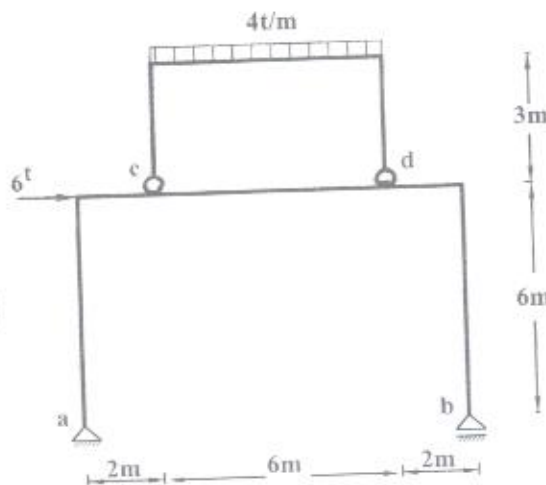


Fig. 3

**4- Problem (4) 15 Marks:**

Using the slope-deflection method, draw the B.M.D. for the given frame of variable I shown in Fig.(4).

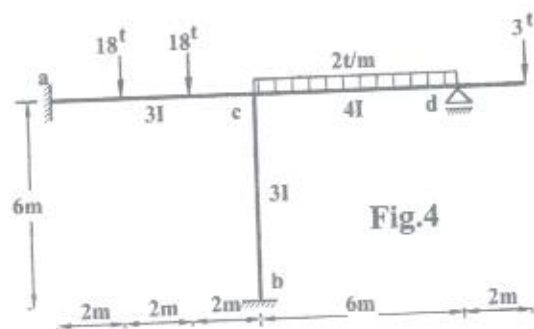


Fig.4

**5- Problem (5) 15 Marks:**

Using the slope-deflection method, draw the B.M.D. for the given frame of constant I shown in Fig.(5).

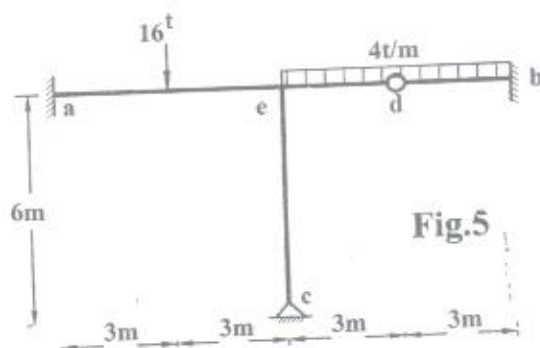


Fig.5

**6- Problem (6) 15 Marks:**

Using the moment distribution method, draw the B.M.D. for the given frame of variable I shown in Fig. (6).

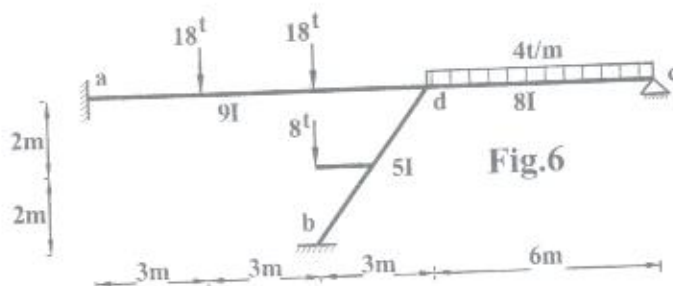


Fig.6

With the best wishes



**Question 2:**

Table (1) shows data given for truss members. By Using St. 52:

- a- It is required to design these separate members (consider their connections as welded ones).
- b- Calculate the required welded lengths.

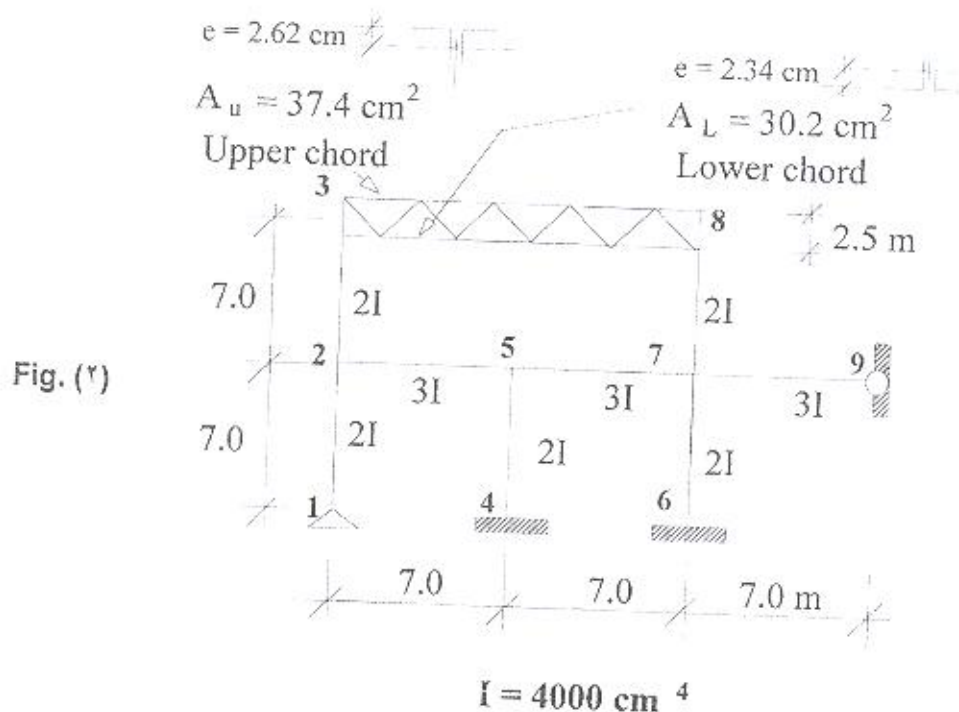
Table (1)

Member	$F_u$ [t]	L [m]	$L_{bx}$ [m]	$L_{by}$ [m]	Type
1	+20	5	-	-	Top chord
2	-15	5	5	2.5	Top chord, Unequal angles
3	+30	4	-	-	Bottom chord
4	0	6	-	-	Diagonal member
5	$\pm 12$	4	4	4	Bottom chord

**Question 3:**

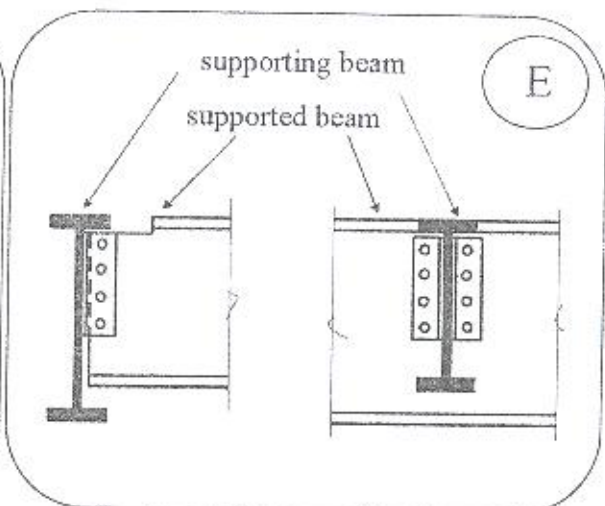
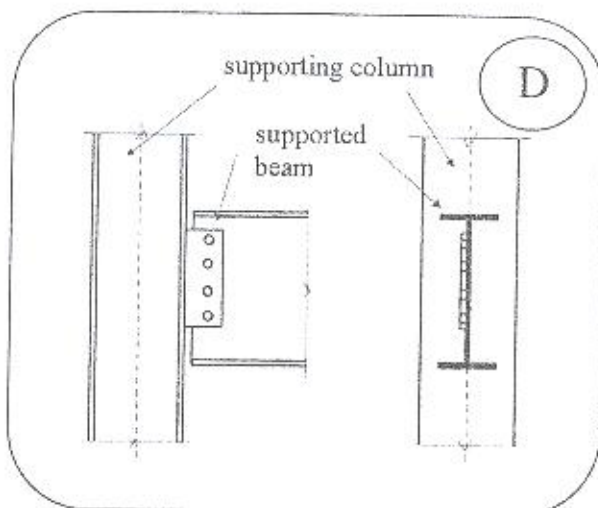
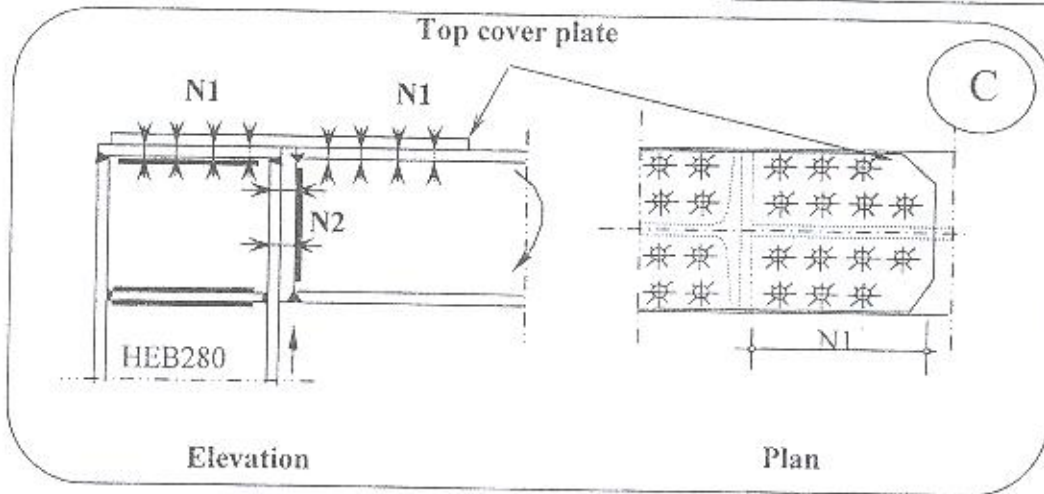
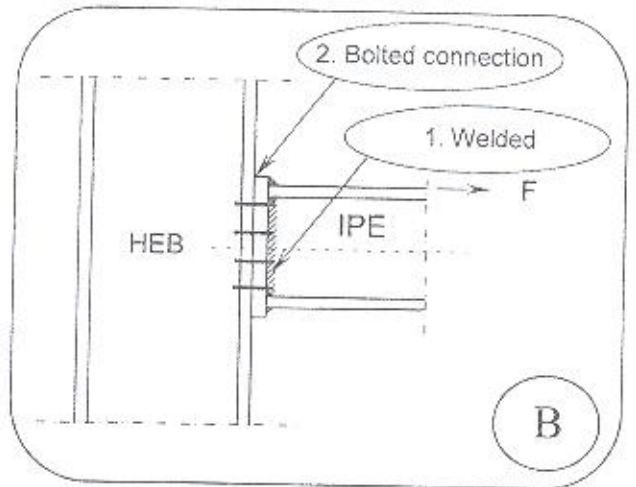
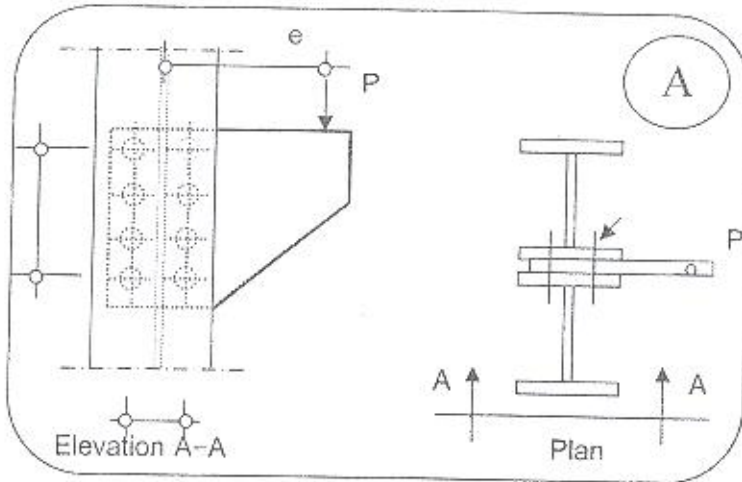
(12 %)

Fig. (2) shows the statical system of a part of an industrial building. According to the Egyptian Code of Practice, compute the effective buckling lengths for columns 1-2, 2-3, 4-5, 6-7 and 7-8.



**Question 4:**

Describe, with net sketches and without any calculations, the straining action of the following steel connections at Figures A, B, C, D and E:



**Question 5:**

it is required to examine the following statements by choosing (✓) or (✗),  
giving the reason for each choice.

1. The theoretical value of the factor (G) for the fixed support is 0.0, while its practical value in design equals to 1.0.
2. The effective buckling length factor (K) for columns in sidesway prevented rigid frames is always greater than 1.0.
3. Among the SIB, IPE and HEB, the best suitable cross-section to be used with columns and as beams subjected to double bending moments is the SIB.
4. Using LRFD method, the strength is considered to equal the nominal or theoretical strength of the member multiplied by a resistance factor, which is nominally less than unity.
5. The vertical and horizontal loads in the plane of the main system are carried by the bracing systems, while the lateral loads are carried by the main system.
6. A tension member with holes can possibly fail by yielding at the net section through holes.
7. For a diagonal zero member with a length of 3ms with a bolted connection ( $\phi=16\text{mm}$ ), the best design cross-section is one angle  $75 \times 75 \times 8$ .
8. The unfactored yielding strength of the tension member which composed of two back-to-back angles  $100 \times 100 \times 10$ , given that the steel is St44, is 107.52t.
9. The minimum angle leg that can be connected using the bolt M24 is  $60 \times 60 \times 6$ .
10. The maximum slenderness ratio ( $\lambda_{\text{max}}$ ) for a long compression member made of St 37 which has a critical buckling stress ( $F_{cr}$ ) of 0.497t/cm<sup>2</sup> is 143.95.

Best wishes, Prof. Dr. Mohamed Dabaon and Exam. Committee



Course Title: Soil Mechanics (3)  
Date: 23 January 2011 (First term)

Course Code: CSE3125  
Allowed time: 3 hrs

Year: 3<sup>rd</sup> Structural Eng.  
No. of Pages: (4)

- Assume any missing data
- Answers should be supported by sketches

Question Number (1) (15 Marks)

- a) What is the main goal of soil exploration? (3 Marks)
- b) What are the different steps of a soil exploration program. (3 Marks)
- c) What are the methods adopted for measuring the field density of soil. (4 Marks)
- d) Discuss with clear sketches the penetration testing of the soil in the field, what are the applications of such tests in engineering practice. (5 Marks)

Question Number (2) (15 Marks)

- a) What are the main differences between consolidation and compaction of soil? (1 Marks)
- b) State the main differences between the standard and the modified proctor tests. (1 Marks)
- c) Summarize with clear sketches some of the geotechnical applications in which the surface compaction can be used. (2 marks)
- d) Describe briefly using clear sketches how to assess the maximum dry density of compacted sand in the field using the sand cone device. (2 Marks)
- e) The following results were obtained from a Proctor test: (6 Marks)

Wc %	4	5	6	8	10	12
$\gamma t/m^3$	1.77	1.79	1.88	1.95	1.91	1.85

It is required to:

- Plot the moisture-dry density curve, and then determine the optimum moisture content "O.M.C" if the specific gravity of the tested soil equals 2.65.
  - Comment on the curve behaviour using the interpretation of the compaction theory.
  - On the same axes, plot the curves of 80% and 90% degrees of saturation.
- f) For the previous problem predict the expected field density of a sub-base layer of the same soil if the supplied field water content is 18% and the required compaction efficiency must not be less than 96%. (3 Marks)

Question Number (3) (15 Marks)

- a) The following Figure (1) shows two cases of retaining walls that are retaining approach fill of a bridge. The approach fill is sand with  $\gamma = 2.0 t/m^3$  and  $\phi'$  of  $33^\circ$ . Calculate the earth pressures distributions on vertical line AB in cases 1 and 2. (5 Marks)

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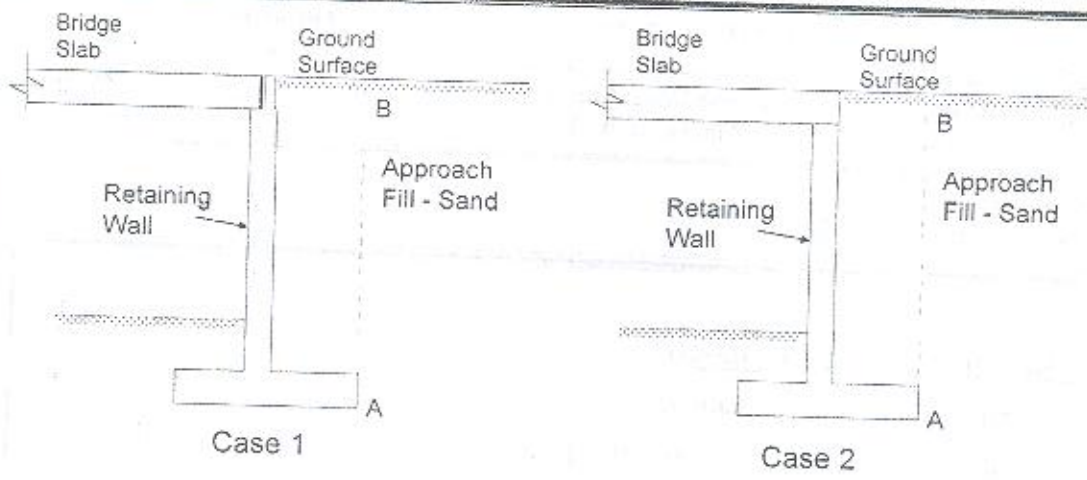


Figure (1)

- b) For the given retaining wall channel (Figure 1), calculate: (10 Marks)
- (I) Factor of Safety against sliding in case of the channel is empty (4 Marks)
  - (II) Factor of safety against overturning in case the channel is full of water (4 Marks)
  - (III) The stress under the base in case the channel is full of water. (2 Marks)

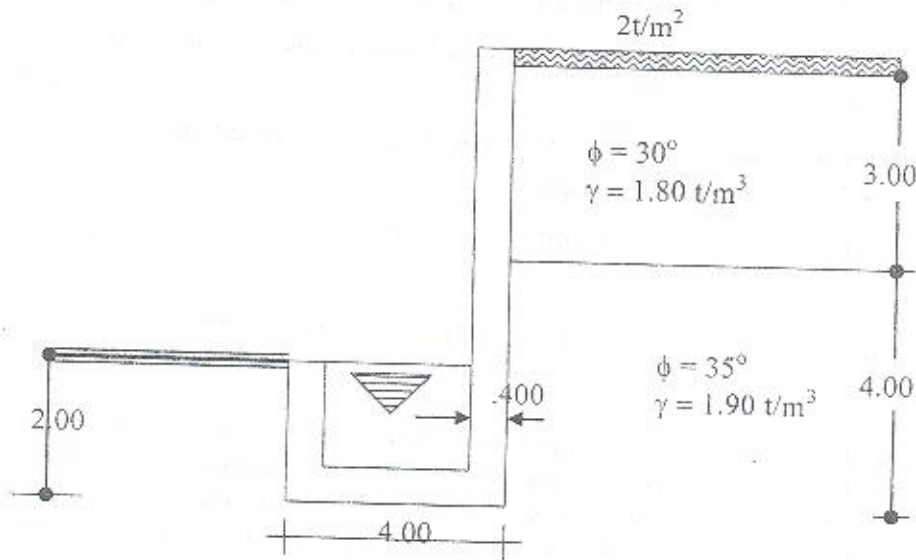


Figure (2)

Question Number (4) (15 Marks)

- a) An infinite slope exists at an angle " $\beta$ " to the horizontal in a clay soil having a unit weight " $\gamma$ " and effective strength parameters " $c'$ " and " $\phi'$ ". Derive an expression for the factor of safety against failure along a shallow slip plane parallel to the ground surface. (3 Marks)

- b) Re-derive the previous expression for the case if a ground water surface exists at the slope surface. (3 Marks)
- c) Use the expression derived in (a) to find the maximum stable slope where  $c' = 0$ ,  $\phi' = 20^\circ$  and  $\gamma = 19 \text{ kN/m}^3$ . (2 Marks)
- d) The factor of safety of the clay cutting shown in Figure (3) is considered inadequate. Hence, in order to increase it, the cross section is to be altered by removing part of the soil as shown. Determine the percentage increase in the factor of safety. (Neglect the tension crack zone). (5 Marks)
- e) If the slope described in (d) is still unsafe, show, using clear sketches, how to protect this slope against failure. (2 Marks)

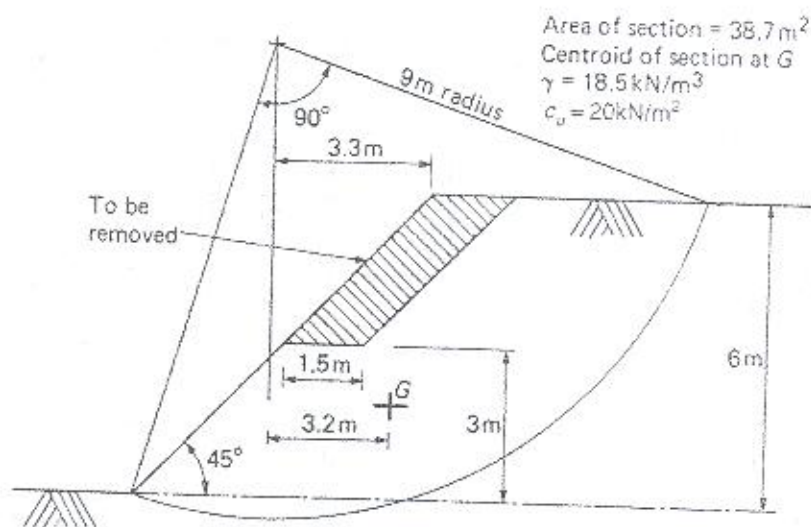


Figure (3)

Question Number (5) (15 Marks)

- a) Explain the effect of the ground water table on bearing capacity of soil in cases of (i) sand (ii) clay (3 Marks)
- b) Explain using sketches the concept of floating foundations (3 Marks)
- c) Consider the site shown in Figure (4). The soil in the site is characterized as sand with unit weight of  $18 \text{ kN/m}^3$ . The groundwater level is at 1.5 m below ground surface. A footing load test is carried out on Footing A ( $1\text{m} \times 1\text{m}$ ) at the ground surface. Based on the test, the ultimate load on Footing A is 290 kN. Calculate the maximum allowable load (P) in kN on Footing B ( $2.5\text{m} \times 2.5\text{m}$ ) in the same site using factor of safety of 3. (9 Marks)

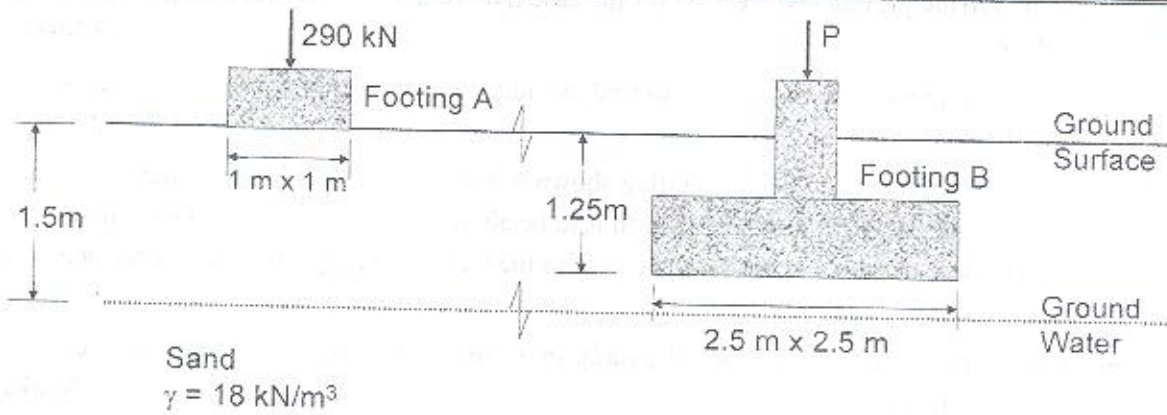


Figure (4)

$\phi^\circ$	$N_c$	$N_q$	$N_\gamma$		$\phi^\circ$	$N_c$	$N_q$	$N_\gamma$
0	5.0	1.0			27.5	25.0	14.0	7.0
5	6.5	1.5			30	30.0	18.0	10.0
10	8.5	2.5	0.5		32.5	37.0	25.0	15.0
15	11.0	4.0	1.0		35	46.0	33.0	23.0
20	15.0	6.5	2.0		37.5	58.0	46.0	34.0
22.5	17.5	8.0	3.0		40	75.0	64.0	53.0
25	20.5	10.5	4.5		42.5	99.0	92.0	83.0

Bearing Capacity Factors from the Egyptian Code of Practice – Shallow Foundations

*Best Wishes.....*

**Course Examination Committee**

*Prof. Dr. Mohamed A. Saky*

*Ass. Prof. Dr. Marawan M. Shahien*

*Dr. Ahmed Farouk A. E. K.*



Course Title: Soil-Structure Interaction  
Date: January 25<sup>th</sup> 2011 (First term)

Course Code: CSE3127  
Allowed time: 3 hrs (Term Exam)

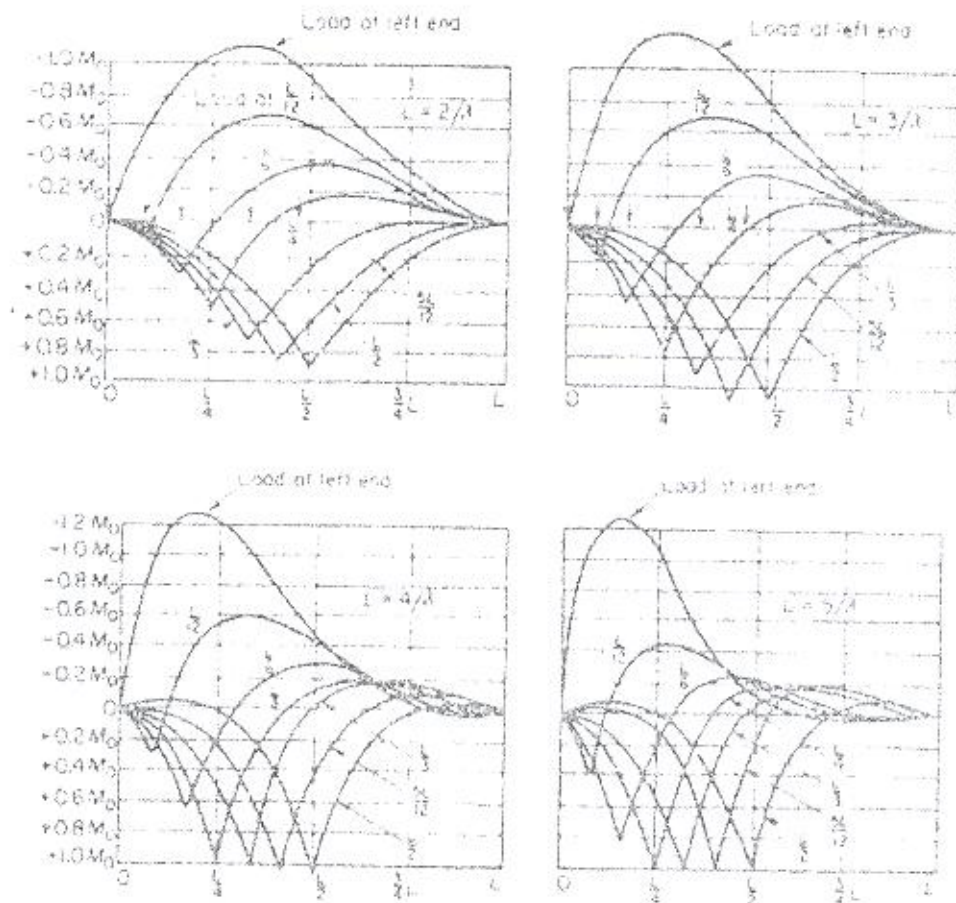
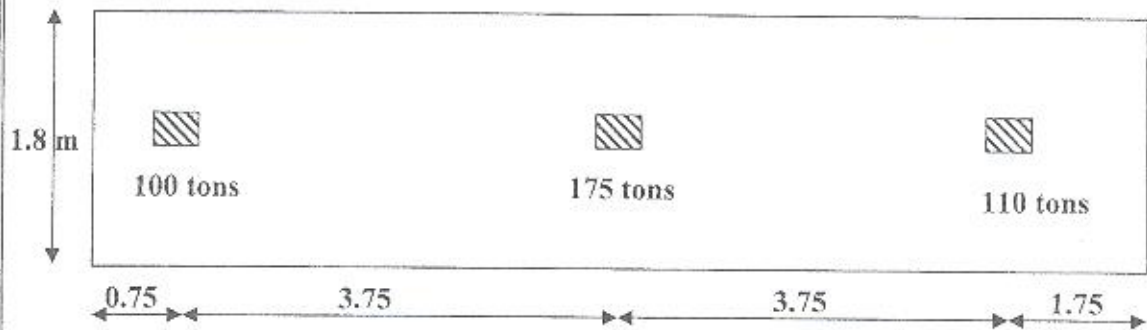
Year: 3<sup>rd</sup>  
No. of Pages: (3)

Answer the following questions ... answers should be supported by sketches

**Problem number (1) (17.0 Marks)**

For the shown combined footing shown in the figure, find out the bending moment using elastic line method. If the following data is known:- the thickness of the footing = 1.25 m

- Elastic modulus of reinforced concrete =  $2 \times 10^6 \text{ t/m}^2$  - Corrected sub grade reaction of soil =  $2.00 \text{ kg/cm}^3$



Note:  $\lambda = \sqrt{\frac{B^3}{4EI}}$



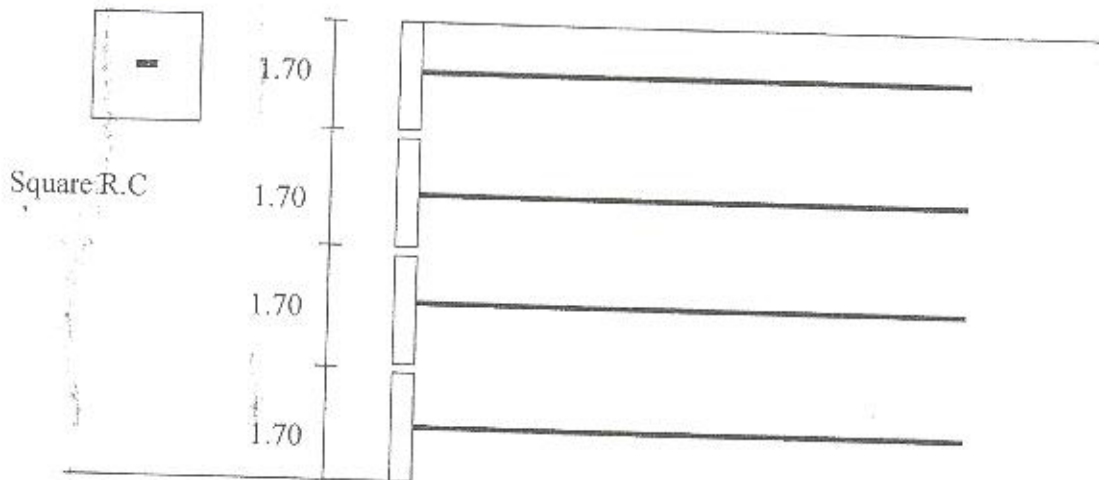
**Problem number (2) (16.5 Marks)**

(a) what are the different material of facing and tie elements used in the reinforced earth retaining walls?

**(4.5 Marks)**

(b) For the shown reinforced earth retaining wall shown in figure 2, find out the required dimensions for ties and design the reinforced concrete facing elements. If the used fill is sand ( $\phi = 30^\circ$ ,  $\gamma = 1.80 \text{ t/m}^3$  and the L.L =  $1.00 \text{ t/m}^2$ ) and ties are made of steel with width 70 mm and allowable stress is  $1400 \text{ kg/cm}^2$

**(12 Marks)**



**Figure 2**

**Question number (3) (9.0 Marks)**

- Discuss the different factors affect the coefficient of subgrade reaction? **(3 Marks)**
- Define the coefficient of subgrade reaction, show the method of estimation its value in field and give the shape of expected relation. **(3 Marks)**
- Using clear sketch show the methods of estimating the total settlement of clayey layer under uniform pressure. **(3 Marks)**

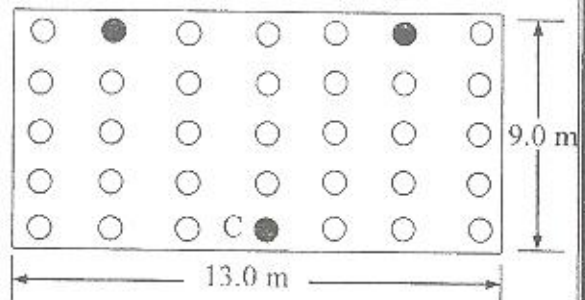
**Problem number (4) (21.0 Marks)**

- a) Using clear sketches, discuss the difference between friction and bearing piles (3 Marks)
- b) Draw the stress distribution along the surface area of friction pile. (3 Marks)

- c) The figure shows the dimensions of a raft A B

foundation over piles for a residential building.

The total load of the structure = 4400 t acting in the left top quarter with  $e_x = 0.3$  m and  $e_y = 0.20$  m. The acting moment on the raft due to considering the lateral loads in y direction = 750 tm. If the pile diameter and spacing are 0.80 m and 2.0 m respectively.

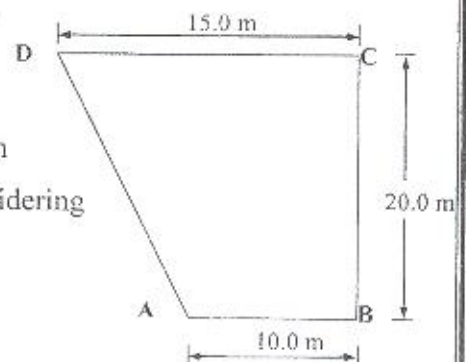


- (i) Determine the maximum load in the piles under vertical loads only. (5 Marks)
- (ii) Determine the loads in the black piles (A, B, and C) under lateral loads. (5 Marks)
- (iii) Determine the maximum and the minimum loads in the piles under vertical loads only if the black piles (A, B and C) were broken during the construction. (5 Marks)

**Problem number (5) (21.5 Marks)**

- a) Using clear sketches, illustrate the difference between the strap and strip footing. (3 Marks)
- b) State how to check the stability of isolated footing subjected to vertical and lateral loads (2.5 Marks)

- c) The figure shows the dimensions of the raft foundation for a residential building. The total load of the structure is 6500 t acting in the right bottom quarter with  $e_x = 0.15$  m and  $e_y = 0.25$  m. The acting moment on the raft due to considering the lateral loads in x direction = 640 tm.



- (consider  $I_{xy}$  for  $\nabla$  is negative)
- (i) Determine the stresses under the raft foundation at point (B), (C) under vertical loads only. (8.0 Marks)
- (ii) Determine the stresses under the raft foundation at point (A), (D) under both vertical and lateral loads (8.0 Marks)

اطيب الأمنيات بالتوفيق

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Course Title: **Theory of plates and shells**  
Date: 25/1/ 2011 (First term)

Course Code: **CSE3130**  
**Term Exam**

Year: 3<sup>rd</sup> year structures  
**Three hours**

**Answer All questions (Assume any missing data and use neat sketches):**

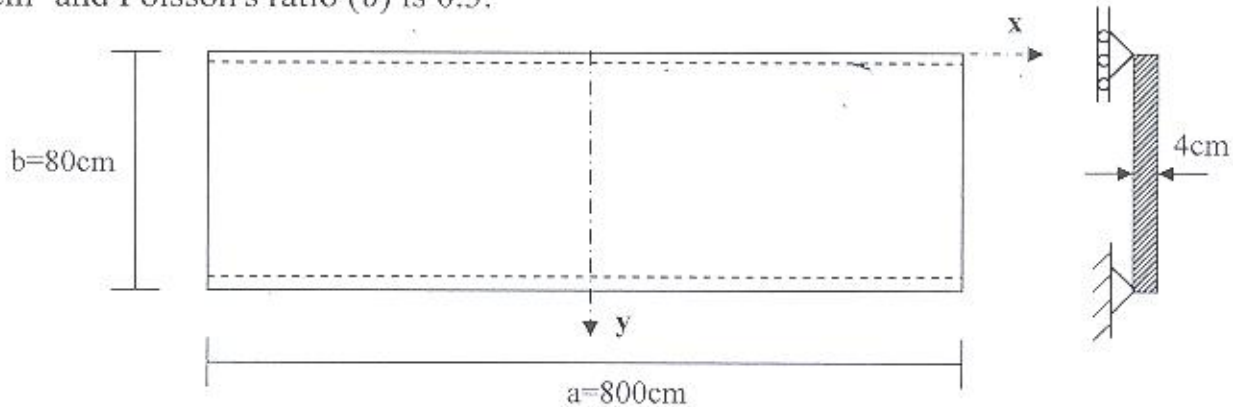
**Q1)**

- What are the main assumptions of small-deflection theory?. Explain briefly each assumption?
- Write down the strain curvature relationships of plate elements?
- Write down the relationships used to calculate the stresses ( $\bar{\sigma}_x$ ,  $\bar{\sigma}_y$  and  $\tau_{xy}$ ) in plate elements?

**Q2)**

- What are the main stress resultants acting on plate elements? Draw a sketch showing these stress resultants?
- Derive the equations used to calculate the stress resultants acting on a plate element using the stresses ( $\bar{\sigma}_x$ ,  $\bar{\sigma}_y$  and  $\tau_{xy}$ )?

**Q3)** Determine the deflection, maximum deflection and maximum bending stresses ( $\bar{\sigma}_x$ ,  $\bar{\sigma}_y$ ) in the shown simply supported plate along the long directions only. The plate is subjected to uniform a sign curve pressure  $p = p_0 \sin (\pi y/b)$ , where  $p_0$  is the peak stress at mid-plate that is equal to  $1.2t/cm^2$ . Assume that the plate is narrow ( $a \gg b$ ). The plate Young's modulus (E) is  $2000t/cm^2$  and Poisson's ratio ( $\nu$ ) is 0.3.



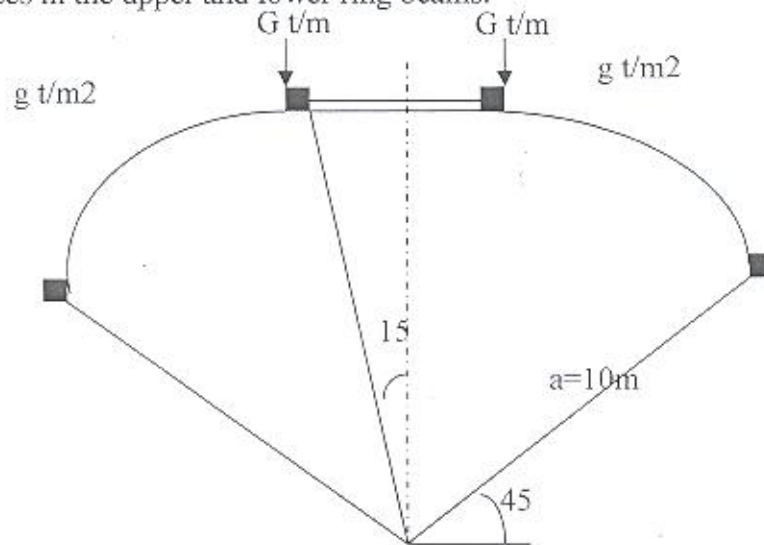
**Q4)** Using the approximate solutions of rectangular plates, determine the maximum uniformly distributed load that can be carried by a fully fixed-ended rectangular plate (2m×3m). The plate has a Young's modulus (E) of  $2000t/cm^2$ , Poisson's ratio ( $\nu$ ) is 0.3, yield stress ( $\bar{\sigma}_y$ ) of  $3.6 t/cm^2$  and has a thickness of 5cm. Determine the maximum deflection corresponding to that load?

Given:  $M_{bc} = (-1/12)pb^2/(1+\alpha^4)$ ,  $M_{bc} = (1/8)pb^2/(3+4\alpha^4)$ ,  $M_{ac} = (-1/24)pb^2$ ,  
 $M_{ac} = 0.009pb^2(1+2\alpha^2 + \alpha^4)$  and  $w_{max} = 0.032(1-\nu^2)pb^4/[(1+\alpha^4)(Et^3)]$

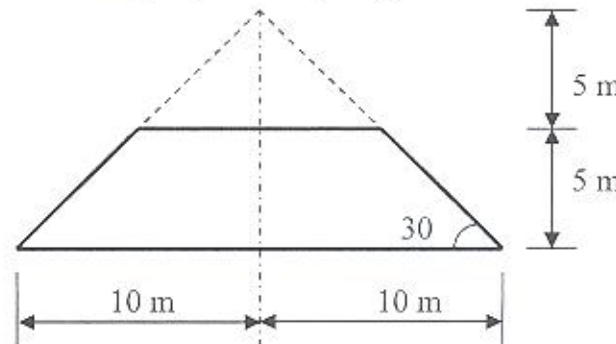
Q5) a) Drive the expression of  $N_\phi = \frac{-1}{r_2 \sin^2 \phi} \left[ \int r_1 r_2 (P_r \cos \phi + P_\phi \sin \phi) \sin \phi d\phi + C \right]$

b) The hall shown in Figure (1) is covered by opened spherical shell of radius of 10.0 m with the dimensions shown in the Figure.

- 1- Calculate and draw the stress resultants in the spherical shell ( $N_\phi$  and  $N_\theta$ ) due to own weight  $g = 0.5$  t per square meter of surface area and the load  $G=2$  t/m.
- 2- Calculate the forces in the upper and lower ring beams.



Q6) For the conical shell shown in Figure, it is required to calculate and draw the stress resultants in the spherical shell ( $N_s$  and  $N_\theta$ ) due to own weight  $g = 0.5$  t per square meter of surface area



$$N_\phi = \frac{-1}{r_2 \sin^2 \phi} \left[ \int r_1 r_2 (P_r \cos \phi + P_\phi \sin \phi) \sin \phi d\phi + C \right]$$

$$\frac{N_\phi}{r_1} + \frac{N_\theta}{r_2} = -P_r$$

$$N_s = \frac{-1}{S \sin \phi} \left[ \int (P_r \cos \phi + P_\phi \sin \phi) S ds + c \right]$$

P.T.O.



جامعة طنطا

الاسئلة / ١١٢٤٧

قسم هندسة الانشاءات  
مجموع الدرجات: ٨٥ درجة



كلية الهندسة

الفرقة الثالثة  
عدد الصفحات: ٤

كود المقرر: CSE3126  
الزمن: ٣ ساعات

عنوان المقرر: حساب الكميات و اعداد المواصفات  
التاريخ: ٢٠١١/١/٢٧ (الترم الاول)

اجب عن جميع الاسئلة، كل سؤال في صفحات متتالية، افترض اي بيانات ناقصة بقيم معقولة.

السؤال الاول: (٢٠ درجة)

- أ- ضع علامة (✓) امام العبارات الصحيحة وعلامة (×) امام العبارات الخاطئة:
١. يقوم مهندس المالك بعمل مقايسة تئمينية محدد بها التكلفة المراد عمل المشروع بها
  ٢. يقوم مهندس المقاول بتقديم مقايسة كمية فعلية في نهاية المشروع فقط
  ٣. عند حساب كمية الحفر بواسطة مهندس المالك يجب ان يحدد ميل جوانب الحفر تبعا لنوع التربة
  ٤. يتم حساب كمية الحفر في المقايسة التئمينية هندسيا من الرسومات
  ٥. حساب كمية الردم للمالك = حساب كمية الردم للمقاول
  ٦. الانتفاش هو زيادة حجم التربة نتيجة لتفككها بعد حفرها
  ٧. يمكن الردم باى نوع من انواع التربة
  ٨. من الاعمال التى تقاس بالعدد اعمال التجارة
  ٩. جدول الحصر هو نموذج من نماذج المقايسات التئمينية
  ١٠. يقوم المقاول بالاعلان عن مناقصة او ممارسة عن المشروع

ب- الشكل التالى لاحد خنادق تمديد مواسير الصرف الصحى بطول ١٠٠ متر و يوجد تحت المسورة فرشاة من الخرسانة العادية كما هو موضح بالشكل. مع العلم بان هذا الخط فى تربة غير متماسكة (زاوية ميل جوانب الحفر ميل ١:١). اذا كان منسوب المياه الجوفية (-٠,٦٥).  
المطلوب:

١. اقتراح بنود الاعمال فى دفتر الكميات الازمة لتنفيذ المشروع بالكامل
٢. اختيار وحدة القياس المناسبة لكل بند
٣. حساب كميات الحفر و الردم من وجهة المالك و المقاول على حدة دون عمل مسطاح

